Name (Print)
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## ME 323-Mechanics of Materials <br> Exam \# 1

Date: October 2, 2019 Time: 8:00 - 10:00 PM

## Instructions:

## Circle your instructor's name and your class meeting time.

| Gonzalez | Kokini | Zhao | Pribe |
| :--- | :--- | :--- | :--- |
| 11:30-12:20PM | 12:30-1:20PM | $2: 30-3: 20 \mathrm{PM}$ | $4: 30-5: 20 \mathrm{PM}$ |

The only authorized exam calculator is the TI-30XIIS or the TI-30Xa.
Begin each problem in the space provided on the examination sheets.
Work on one side of each sheet only, with only one problem on a sheet.
Please remember that for you to obtain maximum credit for a problem, you must present your solution clearly. Accordingly,

- coordinate systems must be clearly identified,
- free body diagrams must be shown,
- units must be stated,
- write down clarifying remarks,
- state your assumptions, etc.

If your solution cannot be followed, it will be assumed that it is in error.
When handing in the test, make sure that ALL SHEETS are in the correct sequential order.

Please review and sign the following statement:
Purdue Honor Pledge - "As a Boilermaker pursuing academic excellence, I pledge to be honest and true in all that I do. Accountable together - We are Purdue."
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## PROBLEM \#1 (25 points)

Rigid bracket CAD is supported by elastic rods (1) and (2). A force $P$ is applied downward at point B. Simultaneously, the temperature of rod (2) is changed by an amount $\Delta T$. The temperature of rod (1) is kept constant. The geometry and material properties of rods (1) an (2) are listed in the following table:

|  | Cross-sectional area | Length | Young's modulus | Coefficient of thermal expansion |
| :--- | :---: | :---: | :---: | :---: |
| Rod (1) | $A$ | $L$ | $2 E$ | $\alpha$ |
| Rod (2) | $A$ | $1.5 L$ | $E$ | $\alpha$ |

(a) If the displacement of point C is known to be $10^{-4}$ in. downward, determine the axial force $F_{1}$ carried by rod (1), the axial force $F_{2}$ carried by rod (2), and the applied force $P$.
(b) Using the forces determined in part (a), determine the factor of safety guarding against yielding of pin A if the pin has a cross-sectional area of $0.1 \mathrm{in}^{2}$, shear yield strength $\tau_{\mathrm{Y}}=20 \mathrm{ksi}$, and is connected to the ground by a double-sided connection as shown in Figure 1.b.

Use the following numerical values: $E=30 \times 10^{3} \mathrm{ksi}, \alpha=10^{-6} /{ }^{\circ} \mathrm{F}, L=10 \mathrm{in}, A=2 \mathrm{in}^{2}, \Delta T=-20^{\circ} \mathrm{F}$ (i.e. the temperature of rod (2) is decreased)


Figure 1.a.

$$
u_{D}=-e_{2}
$$



Figure 1.b: Edge view of the pinned connection at A.
(a)


Equal.: $+\left(\sum M_{A}=-4 F_{1} L+3 P L+2 F_{2} L=0\right.$
$\Rightarrow 4 F_{1}-2 F_{2}=3 P \Rightarrow$ indeterminate! $\rightarrow$ positive since (1) elongates
Force-deformation for (1): $\quad c_{1}=v_{c}=\frac{F_{1} L_{1}}{A_{1} E_{1}}+\alpha_{1} \Delta \Delta T_{1} L_{1}=10^{-4} \mathrm{in}$.

$$
\Rightarrow F_{1}=\frac{A(2 E)}{L} e_{1}=\frac{\left(2 \mathrm{in.}^{2}\right)(60000 \mathrm{ksi})}{(10 \mathrm{im.})}\left(10^{-4} \mathrm{in.}\right)=1.2 \mathrm{kips}
$$

Force-terpenature - deformation for (2): $e_{2}=\frac{F_{2} L_{2}}{A_{2} E_{2}}+\alpha_{2} \Delta T_{2} L_{2}$

$$
\begin{equation*}
\Rightarrow e_{2}=1.5 \frac{F_{2} L}{A E}+1.52 \Delta T L \tag{*}
\end{equation*}
$$

Compatibility: Use similar triangles or the small rotation of bracket (AD)
(see sketch of geometry of deformation on previous

$$
\rightarrow \theta \approx \tan \theta=\frac{e_{1}}{4 L}=\frac{-e_{2}}{2 L} \Rightarrow e_{1}=-2 e_{2}(* *)
$$

Solve: P log $(* *)$ into ( $*$ )

$$
\begin{aligned}
& \text { Plug }(* *) \text { into }(*) \\
& \Rightarrow e_{1}=-2\left[1.5 \frac{F_{2} L}{A E}+1.5 \alpha \Delta T L\right]=-3 L\left[\frac{F_{2}}{A E}+\alpha \Delta T\right] \\
& \Rightarrow A E\left(-\frac{e_{1}}{3 L}-\alpha \Delta T\right)=\frac{-1}{6} F_{1}-A E_{\alpha} \Delta T=1.0 \text { kips }
\end{aligned}
$$

$\left(F_{2}>0\right.$, so (2) is in tension!)

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Use $\sum M_{A}=0$ to solve for $P: \quad P=\frac{1}{3}\left(4 F_{1}-2 F_{2}\right)=\frac{14}{15}$ kips $=0.933 \mathrm{kips}$
(b)

We need the reactions at $A \rightarrow \xrightarrow{+} \sum F_{x}=A_{x}+F_{2}=0$

$$
\begin{aligned}
L+\uparrow \Sigma F_{y}= & A_{y}+F_{1}-P=0 \quad \Rightarrow A_{x}=-F_{2}=-1 k_{\text {ip }} \\
& \Rightarrow A_{y}=P-F_{1}=\frac{-4}{15} k_{\text {kips }}=-0.267 \mathrm{kies}
\end{aligned}
$$

The pin at $A$ is in double shear, so we have
where $\bar{A}=\sqrt{A_{x}^{2}+A_{y}^{2}}=1.035$ kips

and $V=\frac{\bar{A}}{2}$
$\Rightarrow$ the shear stress is $\tau=\frac{V}{A_{\text {pin }}}=\frac{\bar{A}}{2 A_{\text {pin }}}=\frac{(1.035 \mathrm{kips})}{2\left(0.1 \mathrm{in.}^{2}\right)}$

$$
\Rightarrow \tau=5.175 \mathrm{ksi}
$$

Factor of safety: FS $=\frac{\tau_{y}}{\tau}=\frac{20 \mathrm{ksi}}{5.175 \mathrm{ksi}}=3.86$
Remember that the strength goes in the numerator of the factor of safety equation
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## PROBLEM \#2 ( 25 points)

A simply supported beam $A B C D$ is subject to a distributed load that varies from $\mathbf{0}$ to $\boldsymbol{w}=4 \mathbf{k N} / \mathbf{m}$ in section AB. Additionally, a point load $\boldsymbol{P}=\mathbf{5} \mathbf{~ k N}$ is applied at D and moment $\boldsymbol{M}=\mathbf{6} \mathbf{~ k N ~ m}$ is applied at C. Construct the shear force and bending moment diagrams for the beam. Mark the critical values of the shear and bending moment on the diagrams.


Figure 2

ABD


Equilibrom Equivalent load: $(4 \mathrm{kN} / \mathrm{m})(3 \mathrm{~m}) / 2=6 \mathrm{kN}$

$$
\begin{aligned}
& +D M_{A}=-(6)(2)+C_{y}(6)-6+(5)(a)=0 \\
& c_{y}=-\frac{27}{6}=-4.5 \mathrm{kN} \\
& +T \Sigma F_{y}=A_{y}-6+(-4.5)+5=0 \quad A_{y}=5.5 k N \\
& \text { Section } A B \\
& \underbrace{\infty-\frac{4}{3} x}_{1-x \rightarrow 1} y_{y}^{-m} m \\
& v(x)=V(0)+\int_{0}^{x}\left(-\frac{4}{3} x\right) d x \\
& =5.5-\frac{2}{3} \frac{x^{2}}{x 2}=5.5-\frac{2}{3} x^{2} \\
& v(3)=5.5-\frac{2}{3}(3)^{2}=-0.5 \mathrm{kN}
\end{aligned}
$$

$V(x)$ crosses the zero live: $V(0)=5.5-\frac{2}{3} x_{0}^{2}=0$

$$
x_{0}=2.872 \mathrm{~m}
$$

$$
\begin{aligned}
& m(x)=m(0)+\int_{0}^{x}\left(5.5-\frac{2}{3} x^{2}\right) d x=0+\left[5.5 x-\frac{2}{3} \frac{x^{3}}{3}\right] \\
& \text { occurs at } x=x_{0}
\end{aligned}
$$

Maximum $M$ occurs at $x=x_{0}$

$$
\begin{aligned}
& \text { maximum } m \text { occurs at } x=x_{0} \\
& m(2.872)=5.5\left(2.872-\frac{2}{9}(2.872)^{3}=10.53 \mathrm{kN}-\mathrm{m}\right. \\
& m(3)=5.5(3)-\frac{2}{9}(3)^{3}=10.5 \mathrm{kN} \cdot \mathrm{~m}
\end{aligned}
$$

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## PROBLEM \#3 (25 points)

A torque $T$ is applied to a gear-shaft system and is transmitted through rigid gears B and C to a fixed end E as shown in Fig. 3(a). The shafts (1) and (2) are tightly fit to each other. Frictionless bearings are used to support the shafts. The geometry and material property of the shafts and gears are listed in the following table.

|  | Size | Length | Shear modulus |
| :---: | :---: | :---: | :---: |
| Shaft (1) | Outer diameter $=2 d$ <br> Inner diameter $=d$ | $L$ | $2 G$ |
| Shaft (2) | Diameter $=d$ | $L$ | $G$ |
| Shaft (3) | Diameter $=d$ | $L / 2$ | $G$ |
| Shaft (4) | Diameter $=d$ | $L$ | $G$ |
| Gear B | Diameter $=1.5 d$ | Negligible | Rigid |
| Gear C | Diameter $=3 d$ | Negligible | Rigid |

(a) Determine the torque carried by each shaft.
(b) Determine the angle of twist at the free ends A and D.
(c) Consider the cross section $a a^{\prime}$ for the shafts (1) and (2), show the magnitude of the shear stress as a function of the distance from the center on Fig. 3(b). Mark the critical values in the diagram.
(d) Consider the points M and N on the cross section $a a^{\prime}$, shown in Fig. 3(c). Sketch the stress states at M and N on the stress elements on Fig. 3(d).

Express all your answers in terms of $d, L, G, T, \pi$.


Fig. 3(a)
$\qquad$
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Fig. 3(b)


Fig. 3(c)


Stress element $N$
Fig. 3(d)
(a): FBD:

$F_{B C}$ (out of page)
Gear $C$
Equallium:
Gear $C$
Equalubrum:

$$
T_{4}=T
$$

Gear B


FIC (intopoge)


D
A


$$
T_{1}+T_{2}+F_{B C} \cdot r_{C}=0, \quad T_{1}+T_{2}=-\frac{T}{r_{B}} \cdot r_{C}
$$

Compatibility condition:

$$
\begin{aligned}
& \text { ndition: } \quad \phi_{1}=\phi_{2}, \quad \phi_{1}=\frac{11 L_{1}}{G_{1} I_{p 1}}, \quad \phi_{2}=\frac{12 L_{1}}{G_{2} I_{p 2}} \\
& I_{p 1}=\frac{\pi \cdot\left(16 d^{4}-d^{4}\right)}{32}=\frac{\pi \cdot 15 d^{4}}{32}
\end{aligned}
$$

$$
\frac{T_{1} \cdot L}{2 G \cdot I_{P 1}}=\frac{T_{2} L}{G \cdot I_{p 2}}, \quad T_{1}=30 T_{2}
$$

Given $\quad T_{1}+T_{2}=-T \cdot \frac{r_{C}}{r_{B}}=-2 T$

$$
T_{1}=-60 T / 31, \quad T_{2}=-\frac{2 T}{31}
$$

(b): Twist angles:

$$
\begin{aligned}
& \phi_{c}-\phi_{E}=\phi_{2}=\frac{T_{2} L_{2}}{G_{2} I_{p 2}}=-\frac{64 T L}{31 G \pi d^{4}} \\
& \phi_{E}=0, \quad \phi_{c}=-\frac{64 T L}{31 G \pi d^{4}}
\end{aligned}
$$

$$
\begin{aligned}
& \phi_{B} r_{B}=-\phi_{C} r_{C}, \quad \phi_{B}=-2 \phi_{C}=-\frac{128 T L}{31 G \pi d^{4}} \\
& \phi_{A}-\phi_{B}=\phi_{4}=\frac{T_{4} L_{4}}{G_{4} T_{4}}=\frac{T L}{G \cdot \frac{\pi d^{4}}{32}} \\
& \phi_{A}
\end{aligned}=\frac{-128 T L}{31 G \pi d^{4}}+\frac{32 T L}{G \pi d^{4}}=\frac{864 T L}{31 G \pi d^{4}} \simeq 27.9 \frac{T L}{G \pi d^{4}} .
$$

(c):

$$
\begin{aligned}
& D_{B}-Q_{D}=\widehat{G_{3} I_{3}}=0 \\
& \phi_{D}=\phi_{B}=-\frac{\mid 28 T L}{31 \pi G^{4}} \simeq-4.1 \frac{T L}{G \pi d^{4}} \\
& \left|T_{2} \cdot \frac{d}{2}\right|-\frac{2 T}{31} \cdot \frac{d}{2}
\end{aligned}
$$

$$
\begin{aligned}
& \varphi_{D}=\Psi_{B}=\frac{\left|T_{2} \cdot \frac{d}{2}\right|}{I_{P 2}}=\frac{\frac{2 T}{31} \cdot \frac{d}{2}}{\frac{\pi d 4}{32}}=\frac{32}{31} \frac{T}{\pi d^{3}} \\
& \left|\left(\tau_{2}\right)_{r=}=\frac{d}{2}\right|=\frac{d}{4} \\
& \left|\left(\tau_{1}\right)_{r}=\frac{d}{2}\right|=\frac{\left|T_{1} \cdot \frac{d}{2}\right|}{I_{P 1}}=\frac{60 T}{\frac{31}{15 \pi d^{4} / 32}}=\frac{64}{31} \cdot \frac{\tau}{\pi d^{3}} \\
& \left|\left(\tau_{1}\right)_{r=d}\right|=\frac{\left|T_{1} \cdot d\right|}{I_{P 1}}=\frac{128}{31} \frac{T}{\pi d^{3}}
\end{aligned}
$$

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shear stress distribution:
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## PROBLEM \#4 (25 Points):

## PART A - 6 points

For each state of plane stress shown below, i.e., for configurations (a) and (b), indicate whether each component of the state of strain is:

```
* = 0 (equal to zero)
* >0 (greater than zero)
* < 0 (less than zero)
```

The material is linear elastic with Poisson's ratio $v(0<v<0.5)$, and the deformations are small.


|  | $(\mathrm{a})$ | $(\mathrm{b})$ |
| :---: | :---: | :---: |
| $\epsilon_{x}$ | $>0$ | $>0$ |
| $\epsilon_{y}$ | $<\rho$ | $<0$ |
| $\epsilon_{z}$ | $>0$ | $<0$ |
| $\gamma_{x y}$ | $<0$ | $=0$ |
| $\gamma_{x z}$ | $=\bigcirc$ | $=0$ |
| $\gamma_{y z}$ | $=0$ | $=\bigcirc$ |

Fill in with ' $=0$ ', '>0', or ' $<0$ '.

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## PROBLEM \#4 (cont.):

## PART B-4 points

A rod is made up of elastic elements 1 and 2 , each having a length $L$ and cross-sectional area $A$. Elements 1 and 2 have the same Young's modulus $E_{1}=E_{2}$ and coefficient of thermal expansion $\alpha_{1}=\alpha_{2}$. Let $F_{1}$ and $F_{2}$ represent the axial load carried by elements 1 and 2 , respectively, when the temperature of element 1 is increased by $\Delta T_{1}>0$ - while the temperature of element 2 is kept constant $\Delta T_{2}=0$.


Circle the correct answer:
(a) $\left|e_{1}\right|>\left|e_{2}\right|$
b) $\left|e_{1}\right|=\left|e_{2}\right|$
c) $\left|e_{1}\right|<\left|e_{2}\right|$

Circle the correct answer:
a) Elastic element 1 is under tension
b) Elastic element 1 is under compression
(c) $)$ Elastic element 1 is stress-free

Circle the correct answer:


Circle the correct answer:
a) Elastic element 2 is under tension
b) Elastic element 2 is under compression
(c) Elastic element 2 is stress-free

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## PROBLEM \#4 (cont.):

## PART C-4 points

A rod is made up of elastic elements 1 and 2, each having a length $L$ and cross-sectional area $A$. Elements 1 and 2 have the same Young's modulus $E_{1}=E_{2}$ and coefficient of thermal expansion $\alpha_{1}=\alpha_{2}$. Let $F_{1}$ and $F_{2}$ represent the axial load carried by elements 1 and 2 , respectively, when the temperature of element 1 is increased by $\Delta T_{1}>0$ - while the temperature of element 2 is kept constant $\Delta T_{2}=0$.


Circle the correct answer:


Circle the correct answer:
a) Elastic element 1 is under tension
(b) Elastic element 1 is under compression
c) Elastic element 1 is stress-free

Circle the correct answer:


Circle the correct answer:

c) Elastic element 2 is stress-free

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## PROBLEM \#4 (cont.):

## PART D-4 points

A rod is made up of elastic elements 1 and 2, each having a length $L$ and cross-sectional area $A$. Elements 1 and 2 have the same Young's modulus $E_{1}=E_{2}$ and coefficient of thermal expansion $\alpha_{1}=\alpha_{2}$. Let $F_{1}$ and $F_{2}$ represent the axial load carried by elements 1 and 2 , respectively, when the temperature of element 1 is increased by $\Delta T_{1}>0$ - while the temperature of element 2 is kept constant $\Delta T_{2}=0$.


Circle the correct answer:
a) $\left|e_{1}\right|>\left|e_{2}\right|$
(b) $\left|e_{1}\right|=\left|e_{2}\right|$
c) $\left|e_{1}\right|<\left|e_{2}\right|$

Circle the correct answer:
a) Elastic element 1 is under tension
(b) Elastic element 1 is under compression
c) Elastic element 1 is stress-free

Circle the correct answer:
(b) $\begin{aligned} & \left|F_{1}\right|>\left|F_{2}\right| \\ & F_{1}\left|=\left|F_{2}\right|\right.\end{aligned}$
c) $\left|F_{1}\right|<\left|F_{2}\right|$

Circle the correct answer:
a) Elastic element 2 is under tension
b) Elastic element 2 is under compression
c) Elastic element 2 is stress-free
$\qquad$
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PROBLEM \# 4 (cont.):

PART E-7 points
Use only the compatibility condition for the truss structure shown in the figure to find the value of the elongation of member $1\left(e_{1}\right)$ in terms of the elongation of member $2\left(e_{2}\right)$ and the elongation of member 3 ( $e_{3}$ ).
a) Determine an expression for the compatibility condition at A :

$$
e_{1}=\boldsymbol{a} e_{2}+\boldsymbol{b} e_{3}
$$

where $\boldsymbol{a}$ and $\boldsymbol{b}$ are numbers.
Note: Please notice that you are not required to solve for the internal axial forces at equilibrium.
b) the correct answer.

TRUE $\mathrm{pr} \boldsymbol{F A L S E :}$ The elongation of element 1 3.


$$
\left.\begin{array}{ll}
e_{1}=u_{A} \cos \theta_{1}+v_{A} \sin \theta_{1} & \theta_{1}=2 \pi-\arctan (3 / 4) \\
e_{2}=u_{A} \cos \theta_{2}+v_{A} \sin \theta_{2} & v_{1} \\
e_{3}=u_{A} \cos \theta_{3}+v_{A} \sin \theta_{3} & \theta_{2}=\frac{3 \pi}{2}-\arctan (3 / 4) \\
e_{1}=u_{A} \frac{4}{5}-N_{A} \frac{3}{5} \\
e_{2}=-u_{A} \frac{3}{5}-v_{A} \frac{4}{5} \\
e_{3}=v_{A}
\end{array}\right\} \quad \begin{array}{ll}
e_{1}=u_{A} \frac{4}{5}-e_{3} \frac{3}{5} \\
e_{2}=-u_{A} \frac{3}{5}-e_{3} \frac{4}{5} \\
& e_{1}+\frac{4}{3} e_{2}=-e_{3} \frac{3}{5}-e_{3} \frac{16}{15} \\
\hline
\end{array}
$$

