

Name (Print) Solution NB  
(Last) (First)

**ME 323 - Mechanics of Materials**  
**Exam # 1**  
**Date: July 3, 2014 Time: 11:00 – 12:00 PM (in class)**  
**Location: ME 1130**  
**Instructor: Nasir Bilal**

Instructions:

Begin each problem in the space provided on the examination sheets. If additional space is required, use the extra paper provided.

Work on one side of each sheet only, with only one problem on a sheet.

Please remember that for you to obtain maximum credit for a problem, you must present your solution clearly.

Accordingly,

- coordinate systems must be clearly identified,
- free body diagrams must be shown,
- units must be stated,
- write down clarifying remarks,
- state your assumptions, etc.

If your solution cannot be followed, it will be assumed that it is in error.

When handing in the test, make sure that ALL SHEETS are in the correct sequential order.

Remove the staple and restaple, if necessary.

Prob. 1 \_\_\_\_\_  
Prob. 2 \_\_\_\_\_  
Prob. 3 \_\_\_\_\_  
Total \_\_\_\_\_

## Useful Equations

$$\varepsilon_x = \frac{1}{E} [\sigma_x - \nu(\sigma_y + \sigma_z)] + \alpha\Delta T$$

$$\varepsilon_y = \frac{1}{E} [\sigma_y - \nu(\sigma_x + \sigma_z)] + \alpha\Delta T$$

$$\varepsilon_z = \frac{1}{E} [\sigma_z - \nu(\sigma_x + \sigma_y)] + \alpha\Delta T$$

$$\gamma_{xy} = \frac{1}{G} \tau_{xy} \quad \gamma_{xz} = \frac{1}{G} \tau_{xz} \quad \gamma_{yz} = \frac{1}{G} \tau_{yz}$$

$$G = \frac{E}{2(1+\nu)}$$

$$\tau = \frac{Tr}{I_p}$$

$$\phi = \frac{TL}{GI_p}$$

$$I_p = \frac{\pi d^4}{32} \quad \text{Polar moment of inertia for solid circular cross section}$$

$$I_p = \frac{\pi d^4}{32} (d_o^4 - d_i^4) \quad \text{Polar moment of inertia for a circular hollow cross section}$$

$$e = \frac{FL}{EA} + L\alpha\Delta T$$

$$e = u \cos \theta + \nu \sin \theta$$

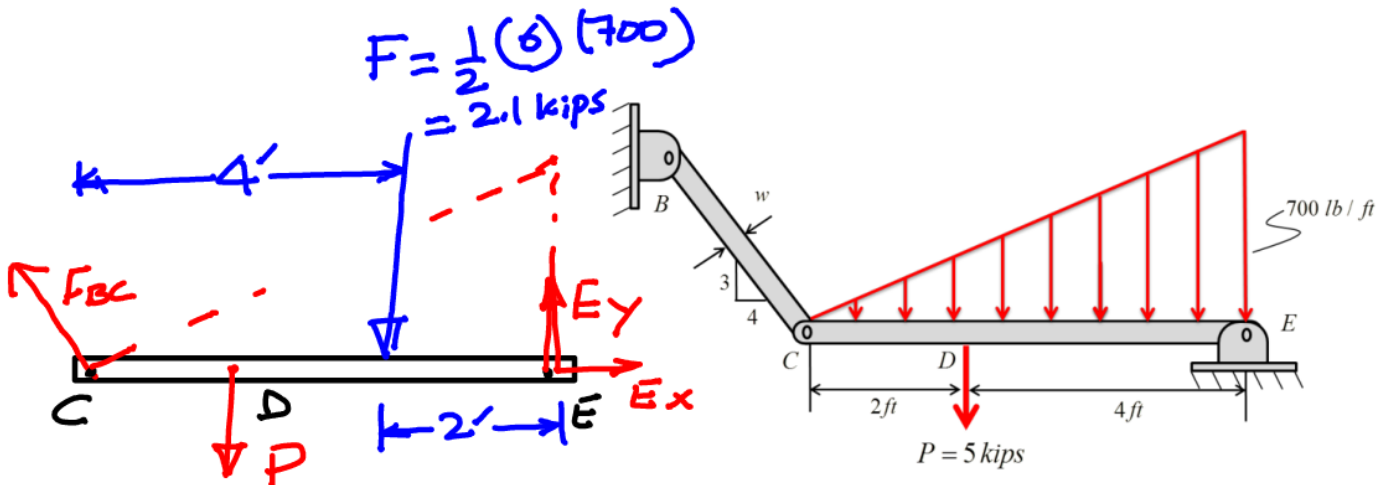
$$\text{Factor of Safety} = \frac{\text{Yield Strength}}{\text{Allowable Stress}}$$

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**PROBLEM #1 (30 points)**

The beam *CDE* is supported by a pin *E* and a rectangular tension bar *BC* also connected to *CDE* by a pin at *C* as shown in figure. The loads acting on the beam are shown in the figure. The tension bar *BC* is to be made of structural steel with a yield strength of  $\sigma_y = 36 \text{ ksi}$ , while pins at *C* and *E* are to be of high-strength steel with a shear yield strength  $\tau_y = 48 \text{ ksi}$ .

- a) If the factor of safety with respect to yielding is  $FS = 3.0$ , and the width of member *BC* is  $w = 1.5 \text{ in.}$ , determine the required thickness,  $t$ , of the tension bar *BC*.
- b) The pin at point *C* is in SINGLE shear and the pin at point *E* is in DOUBLE shear. Determine the required pin diameters at *C* and *E* for the factor of safety  $FS = 3.3$  for shear yielding.



$$\sum M_E = 0 \Rightarrow -F_{BC} \left(\frac{3}{5}\right) (6) + 5(4) + (2.1)(2) = 0$$

$$\Rightarrow \boxed{F_{BC} = 6.72 \text{ kips}}$$

$$+\uparrow \sum F_y = 0$$

$$\Rightarrow \frac{3}{5} F_{BC} - 5 - 2.1 + E_y = 0$$

$$\Rightarrow \boxed{E_y = 3.1 \text{ kips}}$$

$$+\rightarrow \sum F_x = 0 - \frac{4}{5} F_{BC} + C_x = 0$$

$$\Rightarrow \boxed{E_x = +5.38 \text{ kips}}$$

$$E = \sqrt{E_x^2 + E_y^2} = \sqrt{(3.1)^2 + (5.38)^2}$$

$$\boxed{E = 6.209 \text{ kips}}$$

$$\sigma = \frac{FBC}{A_{BC}} = \frac{FBC}{w \cdot t}$$

$$\Rightarrow t = \frac{FBC}{w \cdot \sigma} = \frac{FBC \cdot FS}{w \cdot \sigma_y}$$

$$t = \frac{FBC \cdot FS}{w \cdot \sigma_y}$$

$$= \frac{6.72 \times 10^3 \times 3}{(1.5) \cdot 36 \times 10^3}$$

$$t = 0.3733 \text{ in}$$

For pin C:-

$$\tau = \frac{FBC}{A_{BC}} = \frac{FBC}{\frac{\pi}{4} d^2}$$

$$d = \sqrt{\frac{4 FBC}{\pi \cdot \tau}} = \sqrt{\frac{4 FBC \cdot FS}{\pi \cdot \tau}}$$

$$= \sqrt{\frac{4 \times 6.72 \times 3.3}{\pi \cdot 48}}$$

$$d = .767 \text{ in}$$

$$\tau = \frac{2 F_E}{\pi d^2}$$

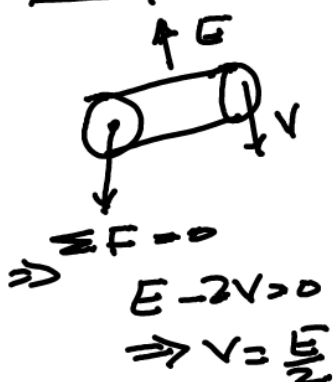
$$d = \sqrt{\frac{2 F_E}{\pi \tau}}$$

$$d = \sqrt{\frac{2 F_E \times FS}{\pi \tau_{allow}}}$$

$$= \sqrt{\frac{2 \times 6.21 \times 10^3 \times 3.3}{\pi \times 48 \times 10^3}}$$

$$d = .521 \text{ in}$$

For pin E:-



$$F_E - 2V = 0$$

$$V = \frac{F_E}{2}$$

$$\tau = \frac{F_E}{2 A_E}$$

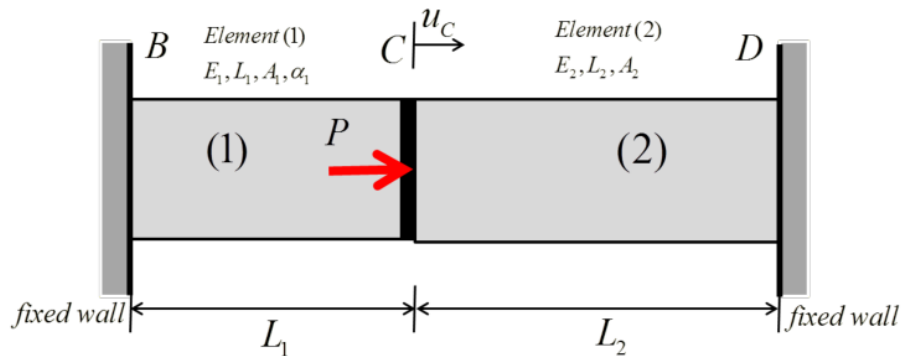
$$\tau = \frac{F_E}{2 \frac{\pi}{4} d^2}$$

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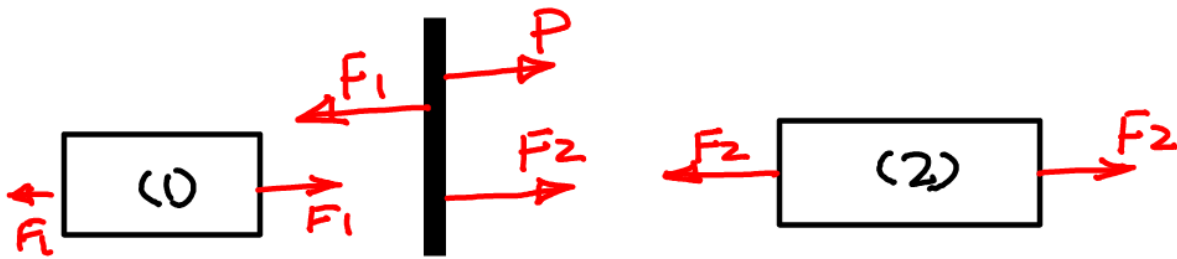
**PROBLEM #2 (30 points)**

A cylindrical rod element (1) is connected to another cylindrical rod element (2) at point C, and both elements are attached at the other end to fixed wall as shown in the figure. A load  $P$  is applied at node C. The temperature of element (1) is INCREASED by  $\Delta T$ , while the temperature of element (2) is NOT changed. Data for the problem are provided in the figure. Determine:

1. the internal forces in elements (1) and (2).
2. the stresses in elements (1) and (2)
3. the horizontal displacement  $u_C$  of point C.



Solution:



Step #01: Equilibrium:

$$\sum F_x = 0 \Rightarrow P + F_2 - F_1 = 0$$

$$\Rightarrow \boxed{F_1 = F_2 + P} \quad (1)$$

Step #02: Force-Deformation Relationship:

$$\boxed{\begin{aligned} e_1 &= f_1 F_1 + \alpha_1 \Delta T L_1 \\ e_2 &= f_2 F_2 \end{aligned}} \quad (2)$$

Step # 03

Compatibility:

$$\begin{array}{l} e_1 + e_2 = 0 \\ e_1 = -e_2 \end{array} \rightarrow (3)$$

$\therefore$

$$f_1 F_1 + \alpha_1 \Delta T L_1 = -f_2 F_2$$

$$F_2 = \frac{-f_1 F_1 + \alpha_1 \Delta T L_1}{f_2}$$

sub in ①

$$F_1 = -\frac{f_1 F_1 + \alpha_1 \Delta T L_1}{f_2} + P$$

$$f_2 F_1 + f_1 F_1 = P f_2 - \alpha_1 \Delta T L_1$$

$$F_1 = \frac{P f_2 - \alpha_1 \Delta T L_1}{f_1 + f_2}$$

where  $f_1 = \frac{L_1}{A_1 E_1}$  ;  $f_2 = \frac{L_2}{A_2 E_2}$

$$F_2 = \frac{P f_2 - \alpha_1 \Delta T L_1}{f_1 + f_2} - P$$

$$= \frac{\cancel{P f_2} - \alpha_1 \Delta T L_1 - P f_1 - \cancel{P f_2}}{f_1 + f_2}$$

$$F_2 = -\frac{(P f_1 + \alpha_1 \Delta T L_1)}{f_1 + f_2}$$

Now sub for  $f_1$  and  $f_2$  i.t.o.  $A, E, L$ .

$$F_1 = \frac{PL_2}{A_2 E_2} - \alpha_1 \Delta T L_1$$

$$\frac{\frac{L_1}{A_1 E_1} + \frac{L_2}{A_2 E_2}}$$

$$F_1 = \frac{(PL_2 - \alpha_1 \Delta T L_1 A_2 E_2) A_1 E_1}{L_1 A_2 E_2 + L_2 A_1 E_1}$$

$$\Rightarrow \sigma_1 = \frac{F_1}{A_1}$$

$$\sigma_1 = \frac{(PL_2 - \alpha_1 \Delta T L_1 A_2 E_2) E_1}{L_1 A_2 E_2 + L_2 A_1 E_1}$$

$$F_2 = - \frac{(PL_1 + \alpha_1 \Delta T L_1)}{\frac{L_1}{A_1 E_1} + \frac{L_2}{A_2 E_2}}$$

$$F_2 = - \frac{(PL_1 + \alpha_1 \Delta T L_1 A_1 E_1) A_2 E_2}{L_1 A_2 E_2 + L_2 A_1 E_1}$$

$$\sigma_2 = \frac{F_2}{A_2} = - \frac{(PL_1 + \alpha_1 \Delta T L_1 A_1 E_1) E_2}{L_1 A_2 E_2 + L_2 A_1 E_1}$$

(ii) Displacement of pt. C.

$$e_c = U_c - U_B$$

$$U_c = f_1 F_1 - \alpha_1 \Delta T L_1$$

$$= \frac{L_1}{A_1 E_1} \left[ \frac{PL_2 - \alpha_1 \Delta T L_1 A_2 E_2}{L_1 A_2 E_2 + L_2 A_1 E_1} \right] A_1 E_1 + \alpha_1 \Delta T L_1$$

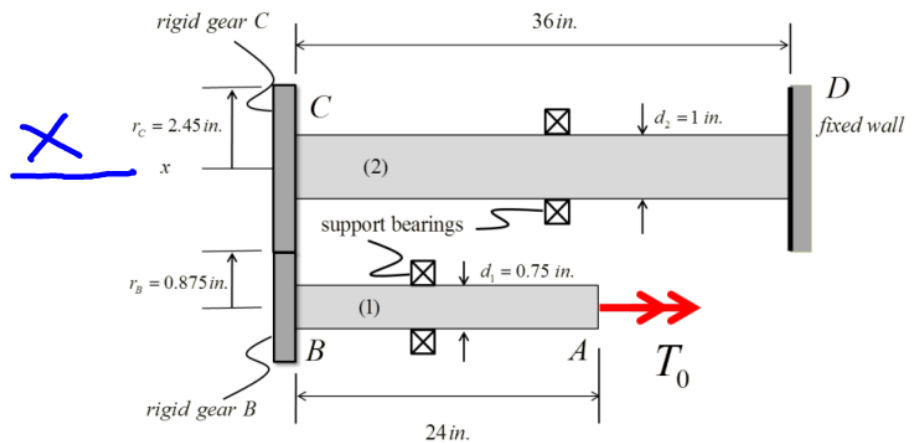
$$U_c = \frac{PL_1 L_2 - \alpha_1 \Delta T L_1^2 A_2 E_2 + \alpha_1 \Delta T L_1}{L_1 A_2 E_2 + L_2 A_1 E_1}$$

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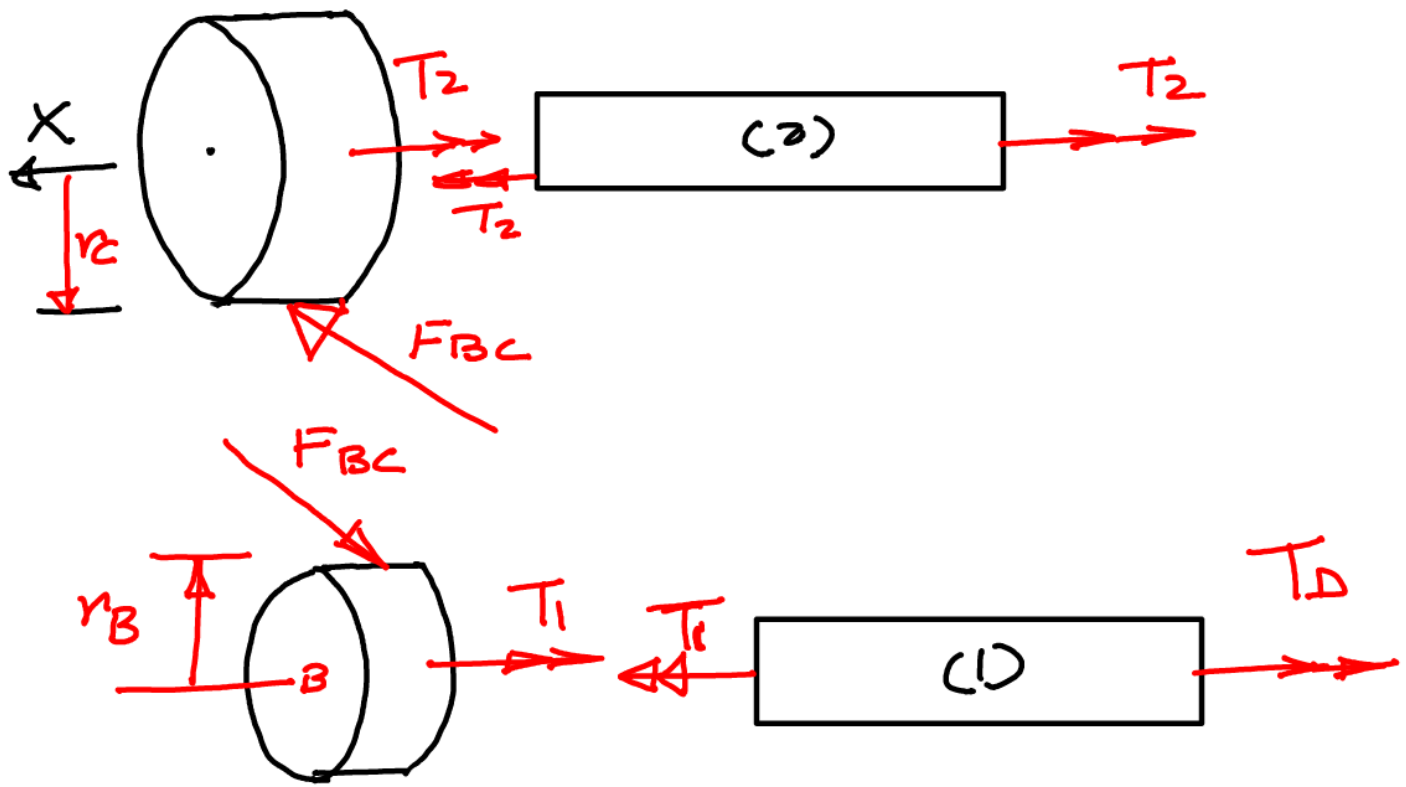
**PROBLEM #3 (40 points)**

Two solid steel shafts are connected by the gears shown in figure. The shear modulus for each shaft,  $G = 11.2 \times 10^6 \text{ psi}$  and the allowable shearing stress is  $8 \text{ ksi}$ . Determine:

1. the largest torque  $T_0$  that may be applied to end A of shaft AB and,
2. the corresponding angle through which end A of shaft AB rotates.



Solution:



$\sum M_x = 0$   
shaft #1:

$T_1 - T_0 = 0 \Rightarrow T_1 = T_0$

$-T_1 - F_{BC} r_B = 0$



$$F_{BC} = \frac{-T_1}{r_B} \longrightarrow (2)$$

$$-F_{BC} \cdot r_C - T_2 = 0$$

$$T_2 = -F_{BC} \cdot r_C \longrightarrow (3)$$

$$(2) \Rightarrow F_{BC} = \frac{-T_0}{r_{BC}}$$

$$T_2 = T_0 \frac{r_C}{r_B} = \frac{2.45}{0.875} T_0$$

$$T_2 = 2.80 T_0$$

Compatibility:

$$\phi_B r_B = -\phi_C r_C$$

$$\phi_B = -\phi_C \frac{r_C}{r_B}$$

$$\phi_B = -2.8 \phi_C$$

Part (1): Max torque.

$$T_1 = T_0$$

$$\tau = \frac{T \cdot r}{J} = \frac{16T}{\pi d^3}$$

$$8000 = \frac{16 T_0}{\pi (0.75)^3}$$

$$\Rightarrow T_0 = 663 \text{ lb-in}$$

For shaft 2 :-

$$(2.8 T_0) = \frac{8000 \pi (1)^3}{16}$$

$$T_0 = 561 \text{ lb-in}$$

The max torque that could be applied is the smaller value of  $T_0$ .

$$T_0 = 561 \text{ lb-in}$$

Part (b) :-

angle of rotation  $\phi_A$ ?

$$\phi_1 = \phi_B - \phi_A \rightarrow \text{(i)}$$

$$\phi_B = -\phi_C \frac{r_C}{r_B} \rightarrow \text{(ii)}$$

$$\phi_2 = \phi_C - \phi_D \rightarrow \text{(iii)}$$

$$\phi_B = -\phi_2 \frac{r_C}{r_B}$$

$$\phi_A = -\phi_1 + \phi_B$$

$$\phi_A = -\phi_1 - \phi_2 \left( \frac{r_C}{r_B} \right)$$

$$\phi_A = -\frac{T_1 L_1}{G J_1} - \frac{T_2 L_2}{G J_2} \left( \frac{2.45}{0.875} \right)$$

$$= \frac{(561)(24)}{11.2 \times 10^6 \times \frac{\pi (0.75)^4}{32}} - \frac{(2.8)(561)(36)}{11.2 \times 10^6 \times \frac{\pi}{32}} \left( \frac{2.45}{0.875} \right)$$

$$\phi_A = -0.1827 \text{ rad} \\ = -10.468 \text{ deg}$$

